To	Mr Stuart Jackson - Myriad	Pages	32
CC			
Subject	121 Link Street and 25 Flierl Street Flooding and Stormwater Assessment		
From	Brett Langtree		
Ref No./File	1613/TM-AR0356	Date	5/06/2026

1.0 INTRODUCTION

Langtree Consulting have been engaged by Myriad to complete a Flooding and Stormwater assessment for the two proposed housing sites. One site is located at 121 Link Road and the other at 25 Flierl Street, Hope Vale.

2.0 THE SITE

The sites are located within Hope Vale. Refer to **Figure 1** below for the site locality.

Figure 1: *Site Locality*

3.0 PROPOSED DEVELOPMENT

It is proposed that the existing dwellings on the sites are to remain with an additional new dwellings being located on both housing sites. Refer to Figure 2 and Figure 3 for the proposed development plans. Refer to Appendix A for the 25 Flierl Street Development Plans and Appendix B for the 121 Link Road Development Plans.

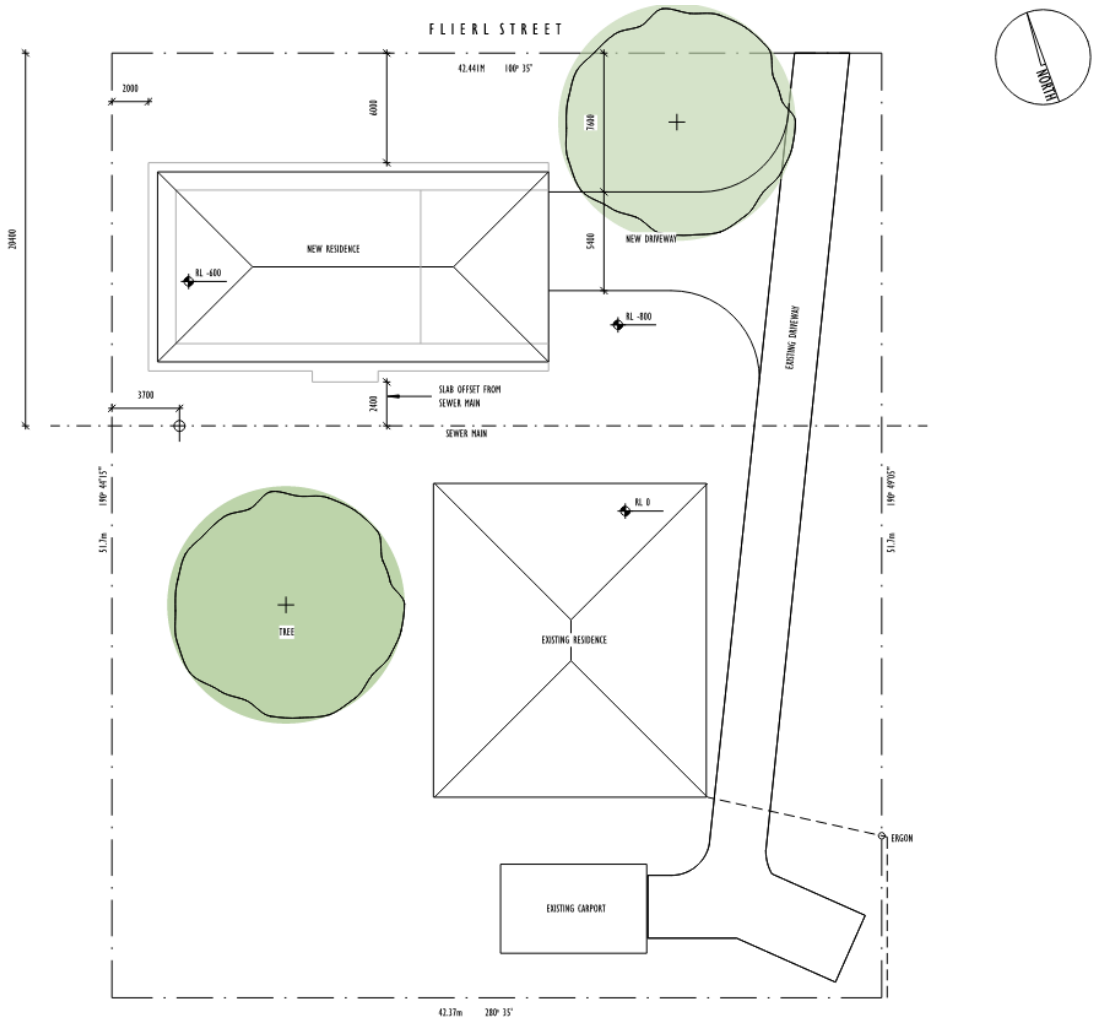


Figure 2: *25 Flierl St Development Plans*

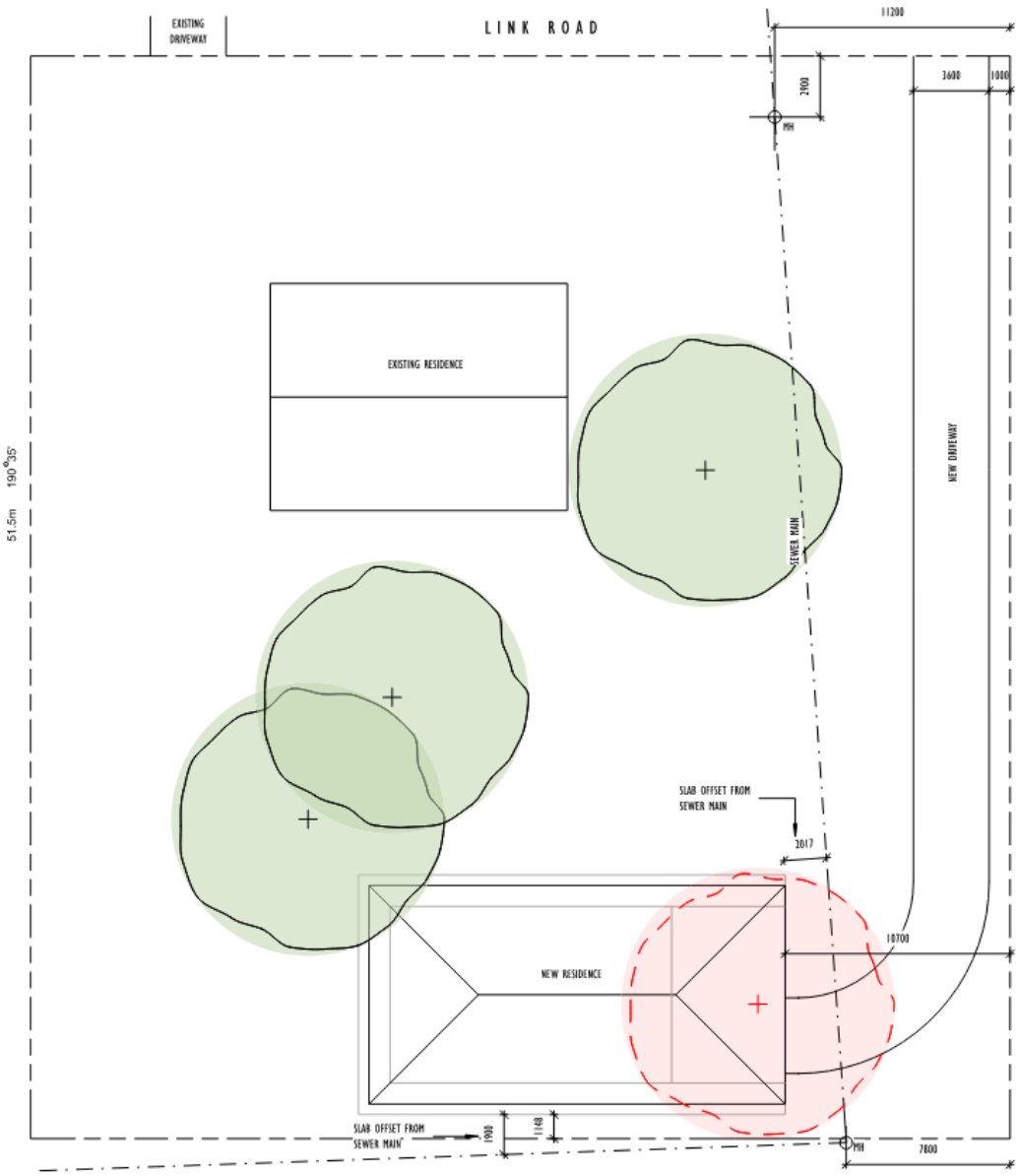


Figure 3: *121 Link Road Development Plans*

4.0 STORMWATER ASSESSMENT

4.1 HYDRAULIC ASSESSMENT

Hydraulic assessment of the site has been undertaken in accordance with the Queensland Urban Drainage Manual (QUDM) 2017. The rational method equations have been adopted:

$$Q_y = (C_y . {}^tI_y . A) / 360$$

Where: Q_y = peak flow rate (m3/s) for average recurrence interval (ARI) of 'y' years

Cy = coefficient of discharge (dimensionless) for ARI of 'y' years

A = area of catchment (Hectares)

${}^tI_y$ = average rainfall intensity (mm/h) for a design duration of 't' hours and an ARI of 'y' years

t = the nominal design storm duration as defined by the time of concentration

4.2 25 FLIERL STREET STORMWATER ASSESSMENT

The site falls from southwest to the northeast of the site. The development catchment area is shown in blue in Figure 4. The catchment area is 2,225m^2 with an impervious area of 250m^2. This catchment discharges to the table drain within Flierl Street. The area within the lot not in the catchment area is not impacted by the proposed development.

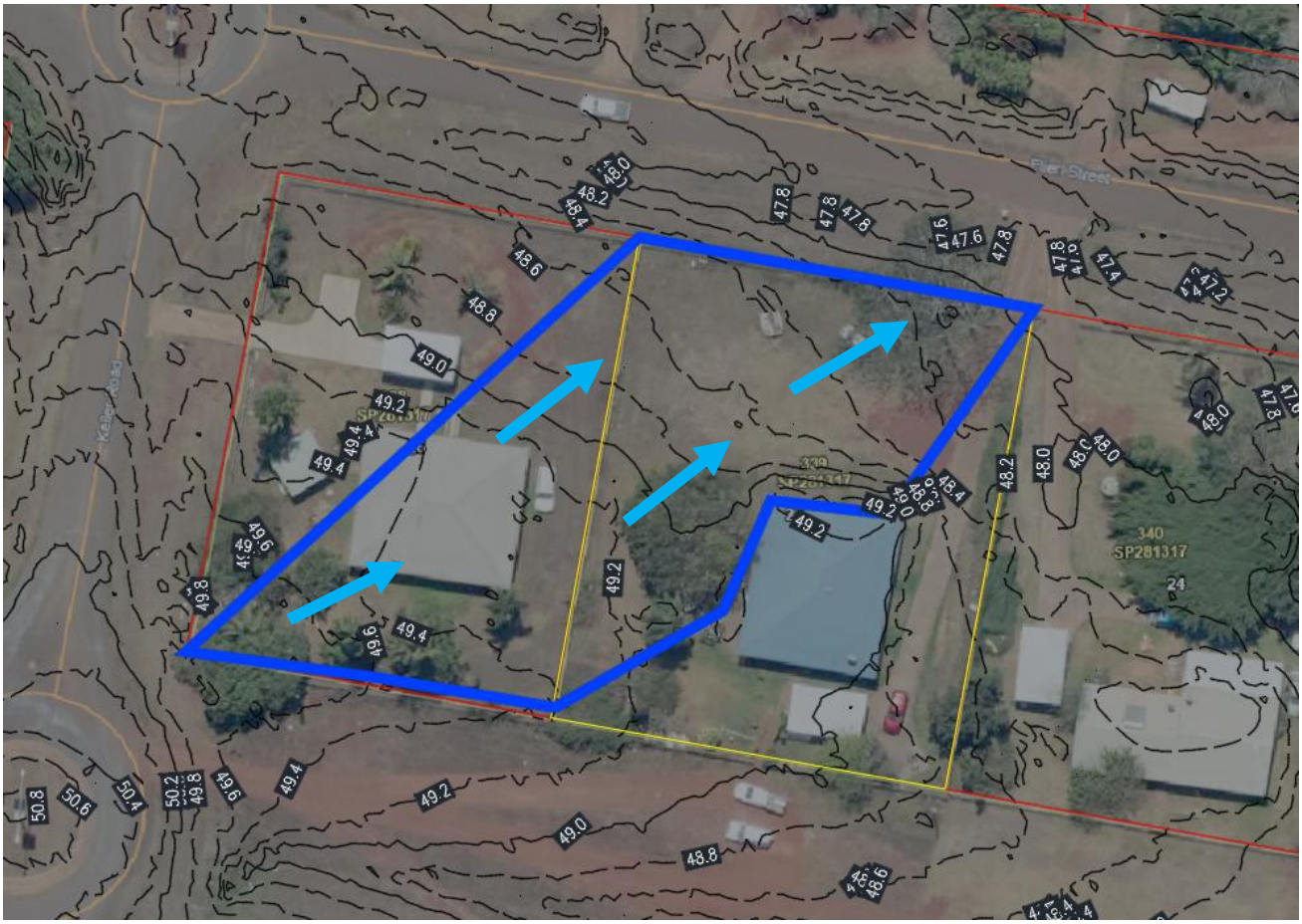


Figure 4: *25 Flierl St Development Catchment Area*

The predevelopment flow is 0.027m^3/s. The impervious area has increased by 328m^2 (total = 578m^2) due to the house and the driveway. Based on this increase the post development flow is 0.028m^3/s which is an increase of 0.001m^3/s. This increase is negligible and as such does not impact the downstream properties. Refer to Appendix C for the calculations.

4.3 121 LINK ROAD STORMWATER ASSESSMENT

The site falls from southwest to the northeast of the site. The predevelopment catchment area is shown in blue in Figure 4Figure 5. The catchment area is 6.7ha with an impervious area of 3,055m^2. This catchment discharges through the properties to the east of the development site.

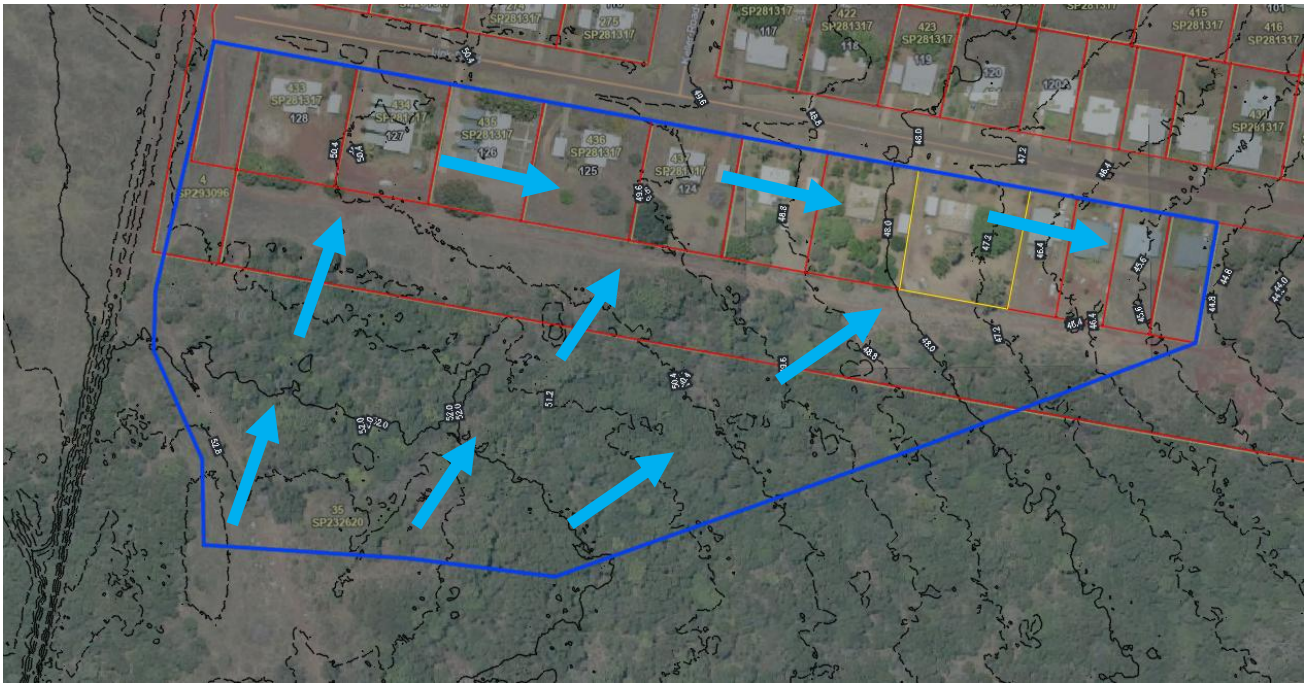


Figure 5: *121 Link Road Catchment Area*

The predevelopment flow is 1.313m^3/s. The impervious area has increased by 417m^2 (total = 3,472m^2) due to the house and the driveway. Based on this increase the post development flow is 1.316m^3/s which is an increase of 0.003m^3/s. This increase is negligible and as such does not impact the downstream properties. Refer to **Appendix D** for the calculations.

5.0 FLOOD ASSESSMENT

Venant Solutions have provided flood levels for both properties. These flood levels have been used to determine the floor levels of the proposed residential dwellings.

As outlined in Venants Flood Advice, 25 Flierl Street is not subject to flooding as it has approximately 50mm depth of overland flow. The flood level at 25 Flierl Street is **48.7m AHD.** Therefore, using a 300mm freeboard the **minimum floor level is 49.0m AHD**. Refer to **Appendix E** for Venants Flood Advice.

As outlined in Venants Flood Advice, 121 Link Road is flooded by a typical depth of 300mm giving a flood level of **48.05m AHD**. Therefore, using a 300mm freeboard the **minimum floor level is 48.35m AHD**. Refer to **Appendix F** for Venants Flood Advice.

6.0 LOCAL STOMRWATER ASSESSMENT

6.1 25 FLIERL STREET

As seen in **Figure 6** below, the stormwater flows directly through the proposed building location. As such the local stormwater is required to be directed around the building via a small bund or drainage swale and into the Flierl Street table drain.

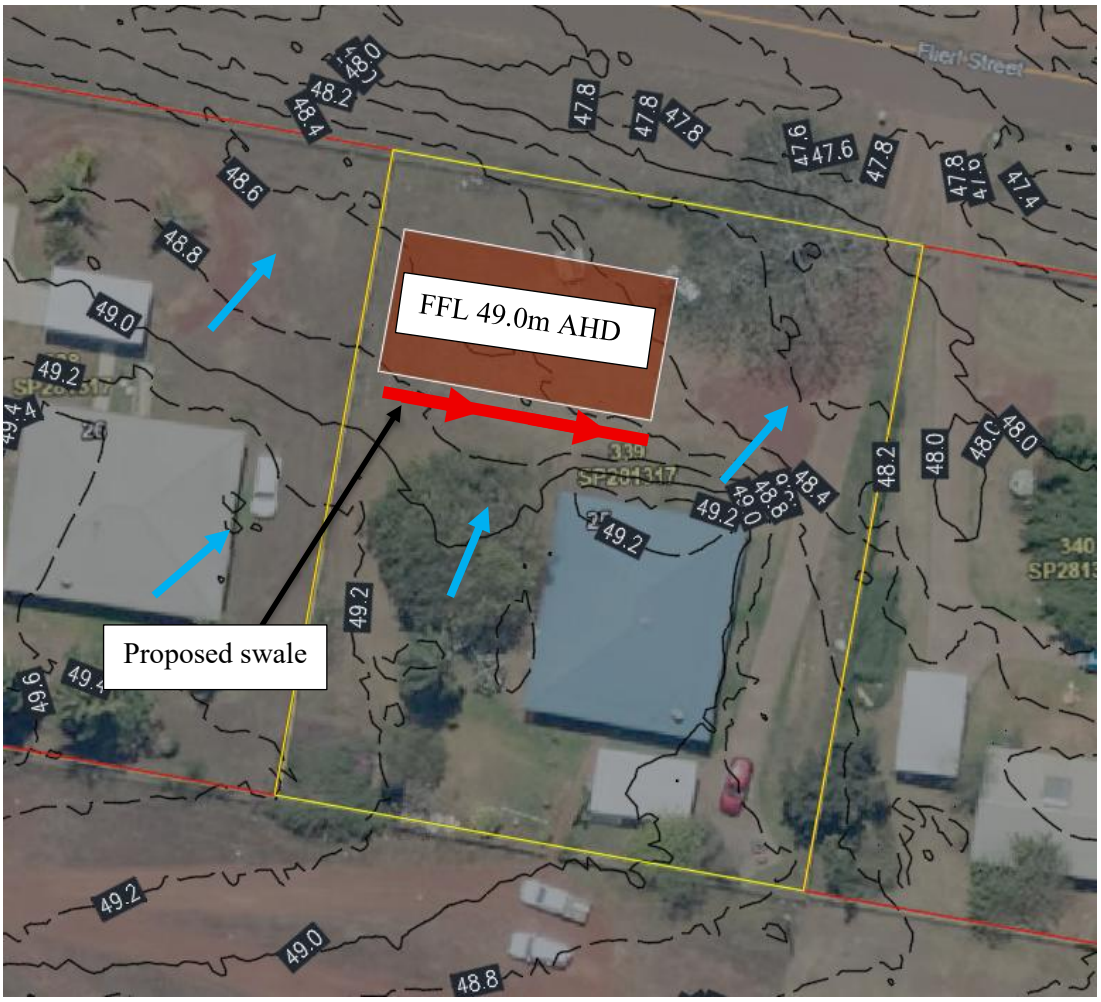


***Figure 6**: 25 Flierl Street Local Stormwater*

6.2 121 LINK ROAD

As seen in **Figure 7**, the stormwater flows directly through the proposed building location. As such the local stormwater is required to be directed around the building via a small bund or drainage swale. They will direct stormwater to the southern side of the lots to the east as the dwellings are on the northern side.

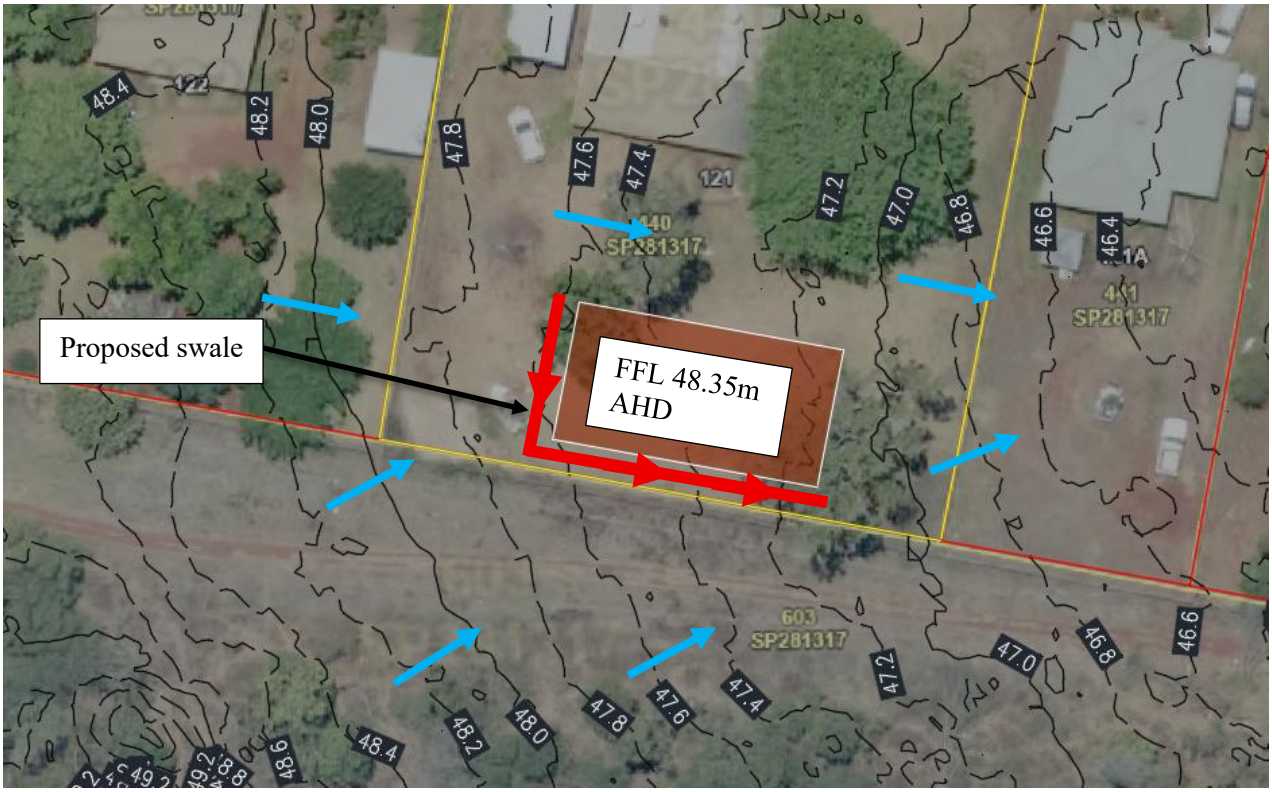


Figure 7: *121 Link Road Local Stormwater*

7.0 SUMMARY

In summary, the two development sites have a negligible impact on surrounding properties in terms or stormwater increase due to the developments. The flood levels have also been checked and the minimum floor levels have been provided.

8.0 CERTIFICATION STATEMENT AND AUTHORISATION

This Engineering Services Report has been prepared under the direction of Brett Langtree (RPEQ No 11932), a civil engineer with 27 years' experience in the planning, design and implementation of civil infrastructure.

……

Brett Langtree – Principal Civil Engineer (RPEQ No 11932), Langtree Consulting

Date: 5 June 2026

APPENDIX A

25 FLIERL STREET DEVELOPMENT PLANS

DRAWING LIST				
SHEET NUMBER	SHEET NAME	SHEET ISSUE DATE	CURRENT REVISION	CURRENT REVISION DESCRIPTION
1.1	COVER PAGE	14.05.26	R1	REVIEW
1.2	GENERAL NOTES	14.05.26		
1.3	PLUMBING NOTES	14.05.26		
1.4	ENGINEERING NOTES	14.05.26		
1.5	ENGINEERS NOTES 2	14.05.26		
2.1	SITE PLAN	14.05.26	R1	REVIEW
3.1	FLOOR PLAN	14.05.26	R1	REVIEW
3.2	DIMENSION PLAN	14.05.26		
3.3	DOOR AND WINDOW NOTES	14.05.26		
4.1	SLAB PLAN	14.05.26		
4.2	SLAB DETAILS	14.05.26		
4.3	SLAB DETAILS	14.05.26		
4.4	BLOCKWALL DETAILS	14.05.26		
5.1	ROOF FRAMING PLAN	14.05.26		
5.2	ROOF DETAILS	14.05.26		
6.1	ELECTRICAL PLAN	14.05.26		
6.2	ELECTRICAL PLAN	14.05.26		
6.3	SCHEMATIC HYDRAULICS PLAN	14.05.26		
7.1	ELEVATIONS	14.05.26	R1	REVIEW
7.3	SECTIONS	14.05.26		
8.1	KITCHEN DETAILS	14.05.26		
8.2	VANITY DETAILS	14.05.26		
9.1	3D VIEWS	14.05.26		

REVIEW

DRAWING STATUS:

REV.	DESCRIPTION	DATE
R1	REVIEW	14.05.26

DRAWN BY:	CHECKED BY:	CHECKED ON:
MYRIAD	STUART JACKSON	14.05.26

myriad
tapitor pty ltd trading as myriad design abn 23 055 558 659
a. 176 buchan street, cairns qld 4870
t. 07 4041 7258 QBSA 1164388

PROJECT NAME:
SECOND DWELLING

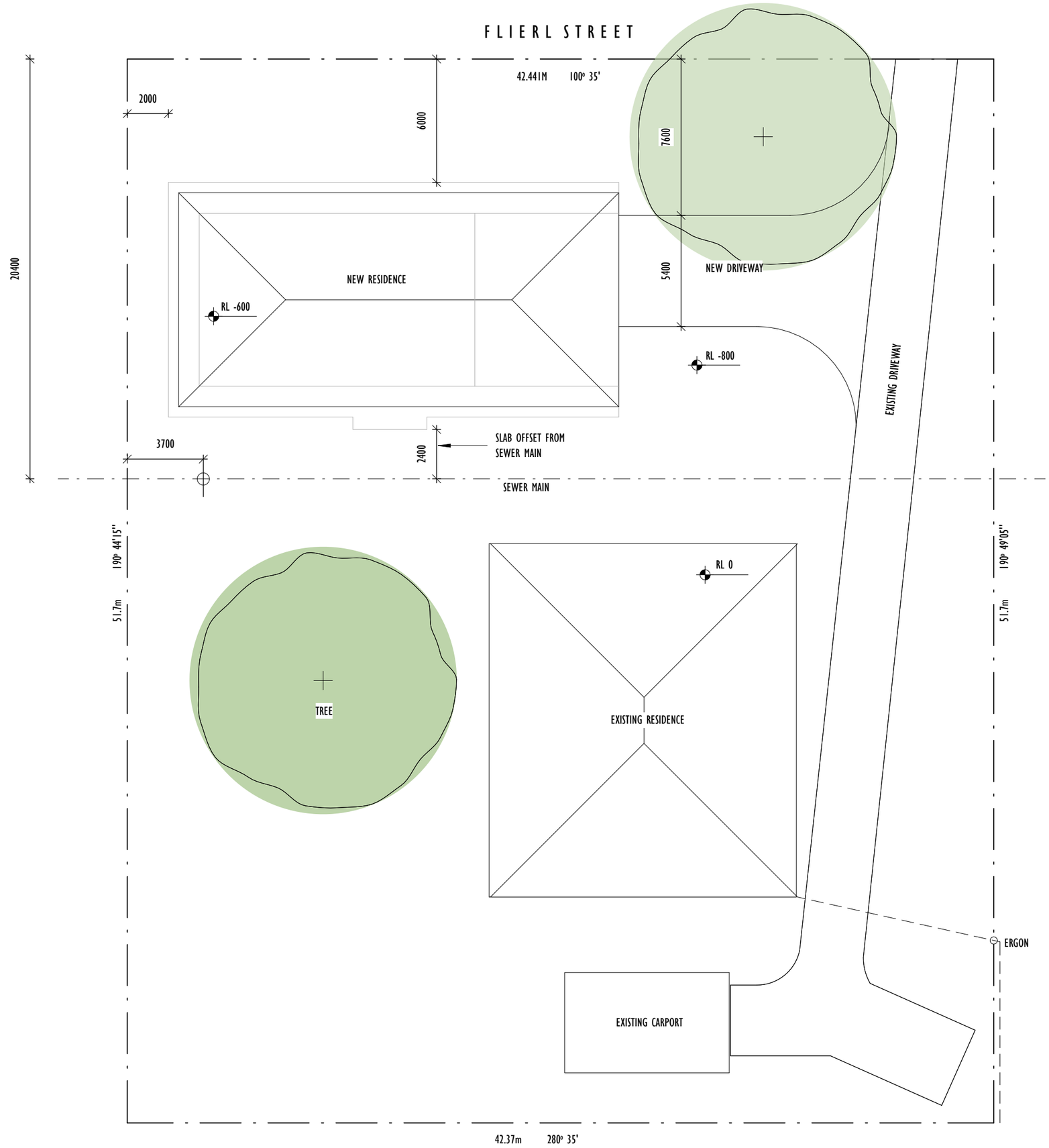
PROJECT ADDRESS:
25 FLIERL ST, HOPEVALE

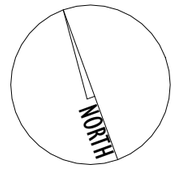
CLIENT:
HVASC

COVER PAGE

SCALE @ A3
ISSUE DATE 14.05.26

SHEET NUMBER **3007 - 1.1**





REVIEW

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DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALING. CONTRACTOR TO VERIFY ALL DIMENSIONS AND APPLIANCES SETOUT ON SITE BEFORE CONSTRUCTION. INCONSISTENCIES SHALL BE REPORTED TO THE DESIGNER IMMEDIATELY. THIS DRAWING IS SUBJECT TO COPYRIGHT AND REMAINS THE PROPERTY OF MYRIAD DESIGN.

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PROJECT ADDRESS:
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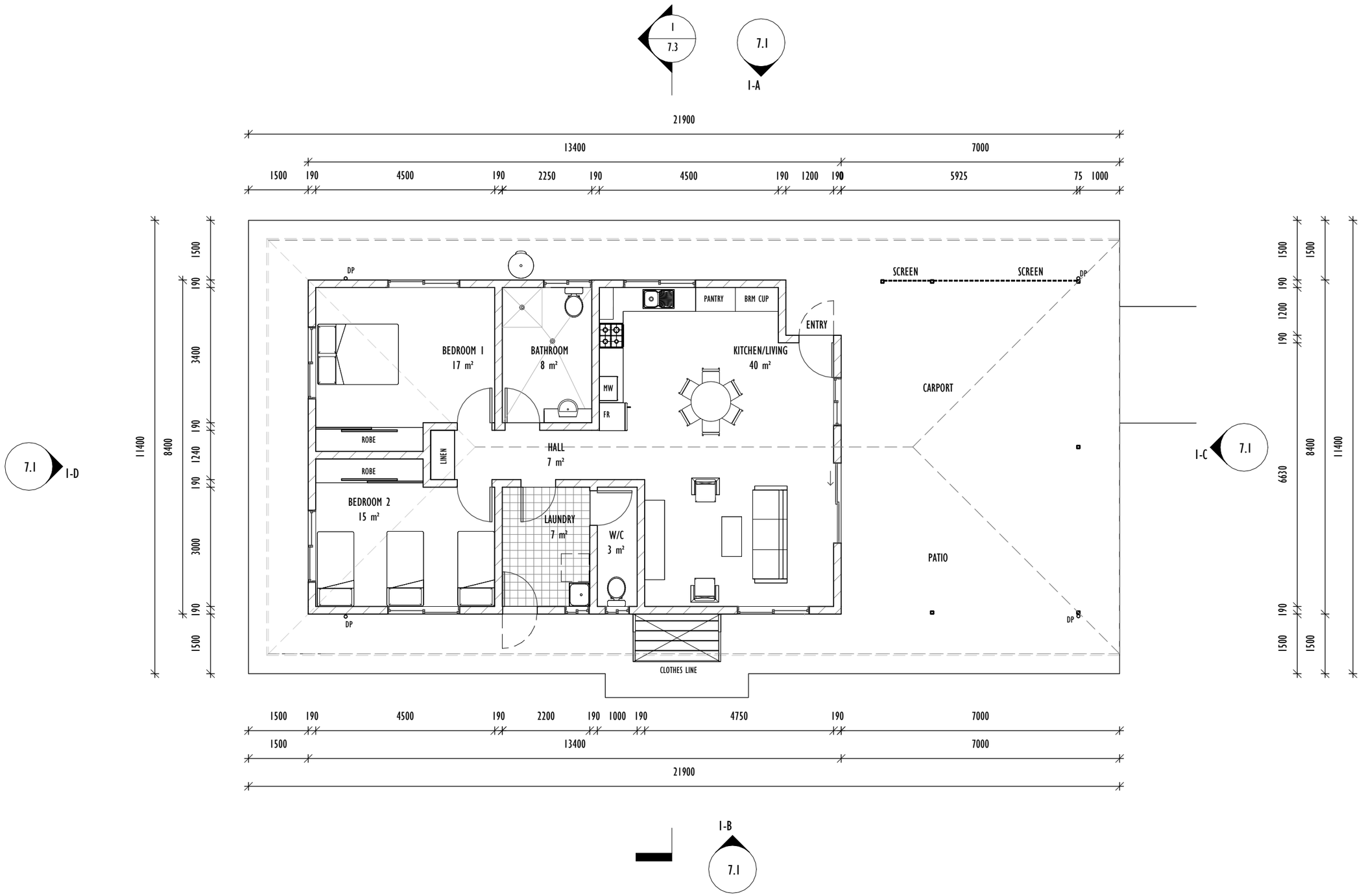
SITE PLAN

SCALE 1 : 200@ A3
ISSUE DATE 14.05.26

SHEET NUMBER 3007 - 2.1



1 / 3.1 **FLOOR PLAN**

1 : 100

REVIEW

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R1	REVIEW	14.05.26

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PROJECT NAME:
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PROJECT ADDRESS:
25 FLIERL ST, HOPEVALE

CLIENT:
HVASC

FLOOR PLAN

SCALE	1 : 100@ A3
ISSUE DATE	14.05.26

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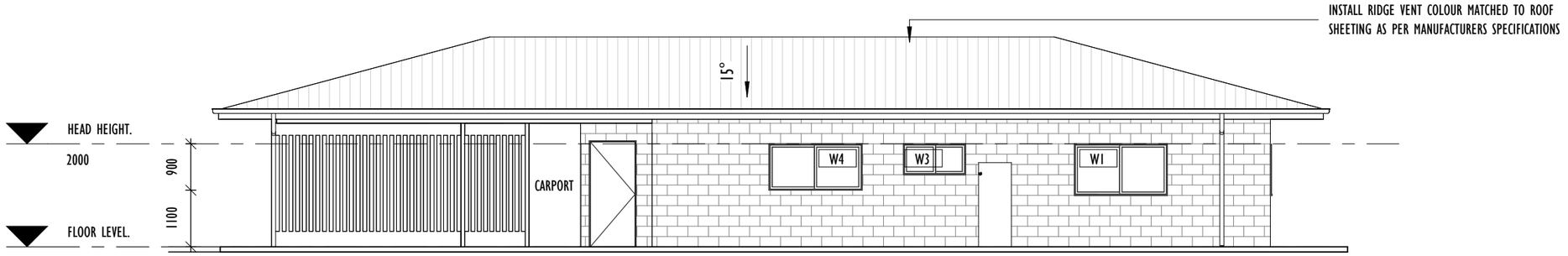

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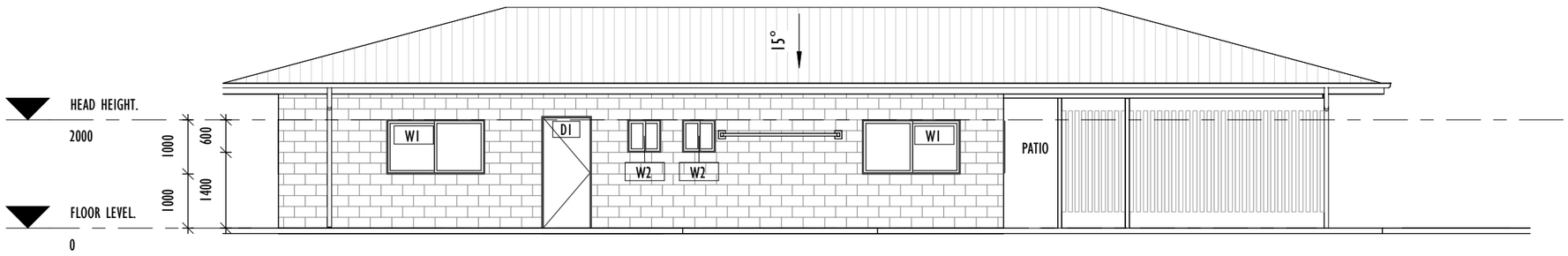

I-B | 3.1 | 7.1

ELEVATION 1-B

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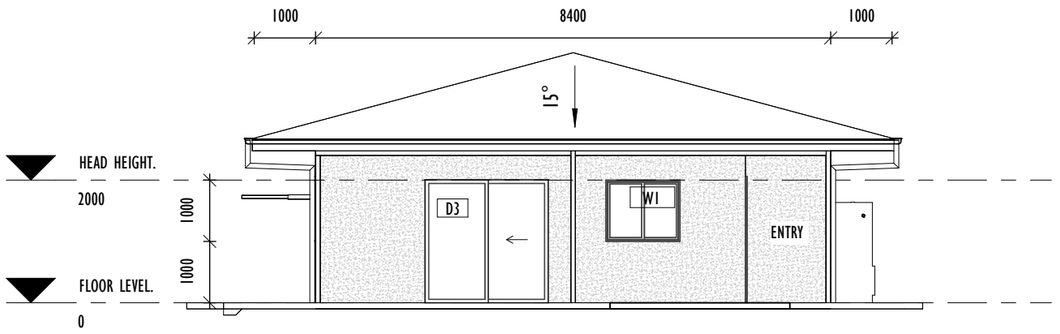

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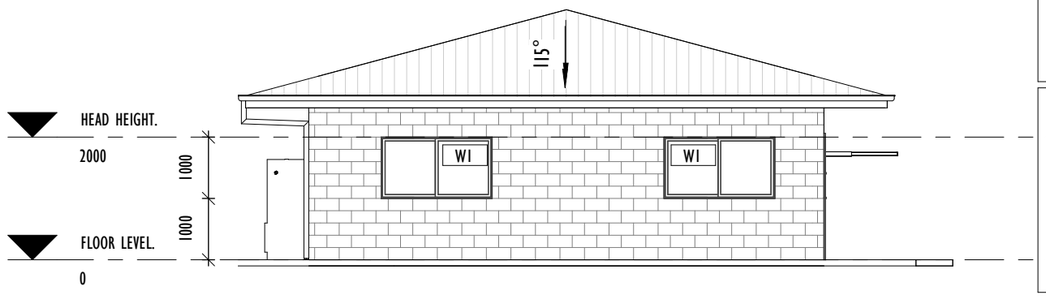

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ELEVATION 1-D

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REVIEW

DRAWING STATUS:

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R1	REVIEW	14.05.26

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PROJECT NAME:
SECOND DWELLING

PROJECT ADDRESS:
25 FLIERL ST, HOPEVALE

CLIENT:
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ELEVATIONS

SCALE 1 : 100@ A3
ISSUE DATE 14.05.26

SHEET NUMBER 3007 - 7.1

APPENDIX B

121 LINK ROAD DEVELOPMENT PLANS

DRAWING LIST				
SHEET NUMBER	SHEET NAME	SHEET ISSUE DATE	CURRENT REVISION	CURRENT REVISION DESCRIPTION
1.1	COVER PAGE	14.05.26	R1	REVIEW
1.2	GENERAL NOTES	14.05.26		
1.3	PLUMBING NOTES	14.05.26		
1.4	ENGINEERING NOTES	14.05.26		
1.5	ENGINEERS NOTES 2	14.05.26		
2.1	SITE PLAN	14.05.26	R1	REVIEW
3.1	FLOOR PLAN	14.05.26	R1	REVIEW
3.2	DIMENSION PLAN	14.05.26		
3.3	DOOR AND WINDOW NOTES	14.05.26		
4.1	SLAB PLAN	14.05.26		
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4.3	SLAB DETAILS	14.05.26		
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5.1	ROOF FRAMING PLAN	14.05.26		
5.2	ROOF DETAILS	14.05.26		
6.1	ELECTRICAL PLAN	14.05.26		
6.2	ELECTRICAL DETAILS AND LEGEND	14.05.26		
6.3	SCHEMATIC HYDRAULICS PLAN	14.05.26		
7.1	ELEVATIONS	14.05.26	R1	REVIEW
7.2	SECTIONS	14.05.26		
8.1	KITCHEN DETAILS	14.05.26		
8.2	VANITY DETAILS	14.05.26		
9.1	3D VIEWS	14.05.26		

REVIEW

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REV.	DESCRIPTION	DATE
R1	REVIEW	14.05.26

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PROJECT NAME:
SECOND DWELLING

PROJECT ADDRESS:
121 LINK RD, HOPEVALE

CLIENT:
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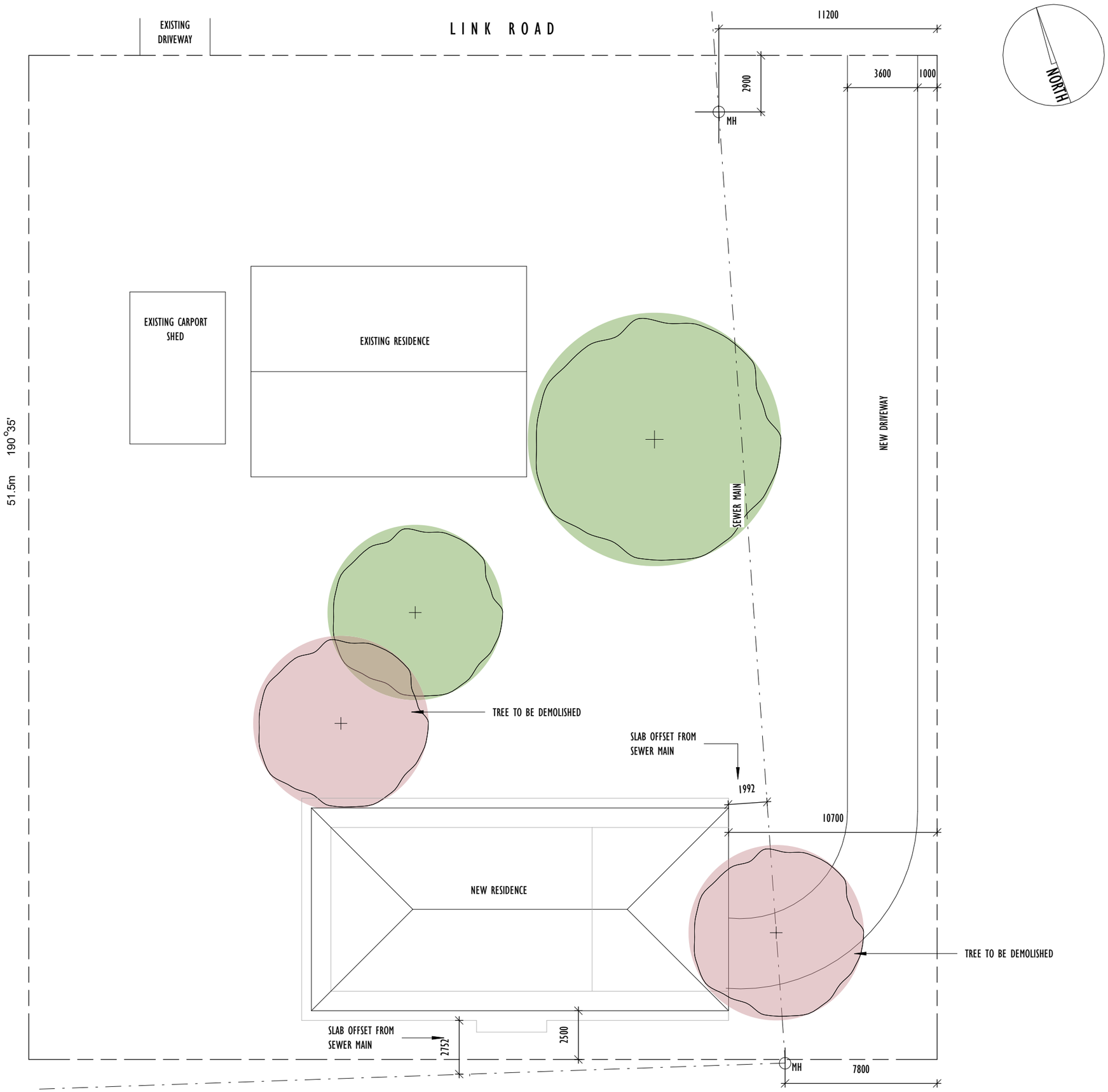
COVER PAGE

SCALE @ A3
ISSUE DATE 14.05.26

SHEET NUMBER **3007 - 1.1**



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t. 07 4041 7258 QBSA 1164388

PROJECT NAME:
SECOND DWELLING

PROJECT ADDRESS:
121 LINK RD, HOPEVALE

CLIENT:
HVASC

SITE PLAN

SCALE 1 : 200@ A3
ISSUE DATE 14.05.26

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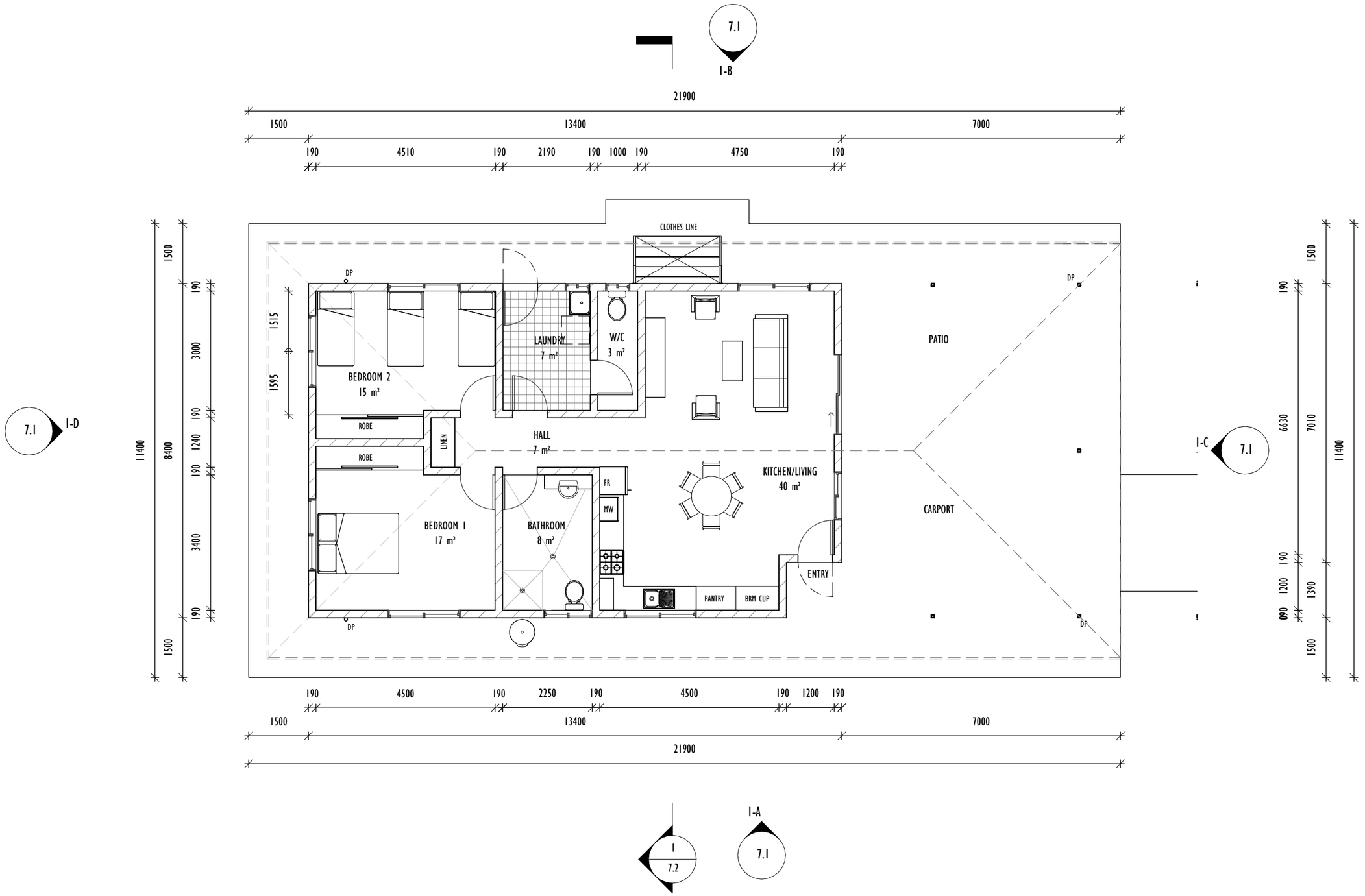

FLOOR PLAN

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REVIEW

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t. 07 4041 7258 QBSA 1164388

PROJECT NAME:
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PROJECT ADDRESS:
121 LINK RD, HOPEVALE

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SCALE	1 : 100@ A3
ISSUE DATE	14.05.26

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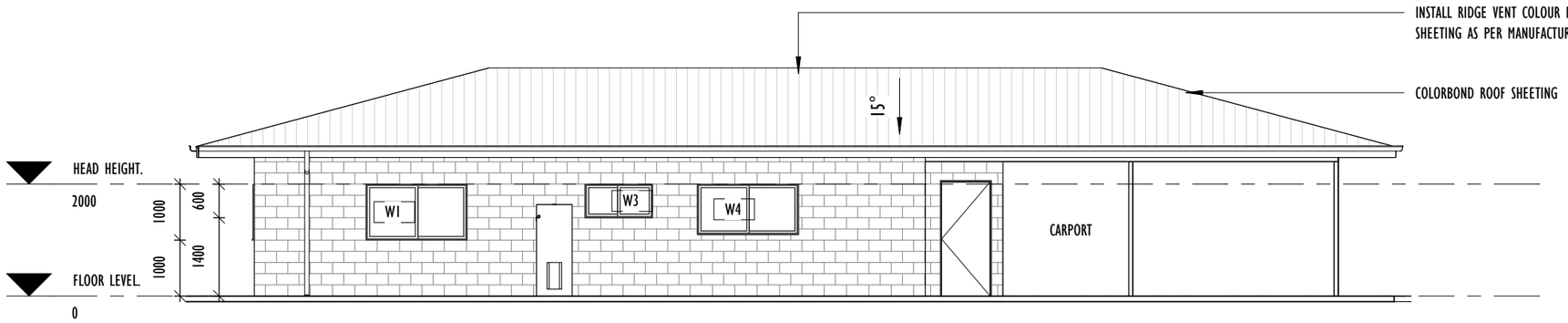

I-A | 3.1 | 7.1

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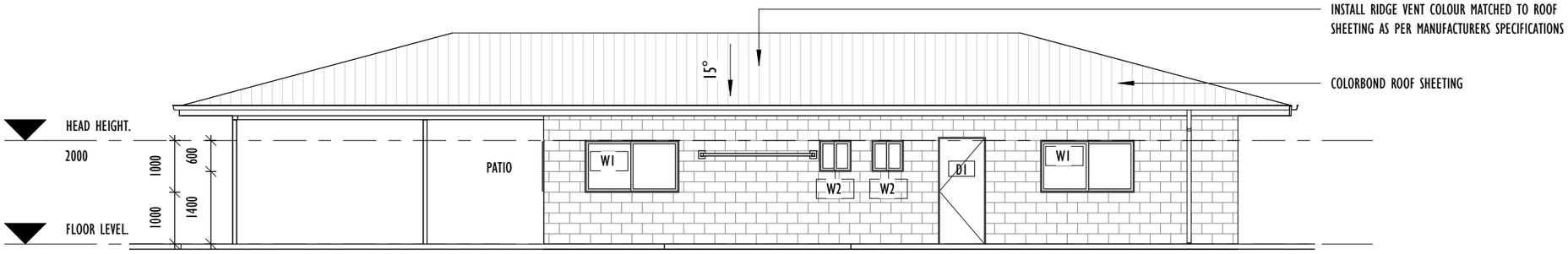

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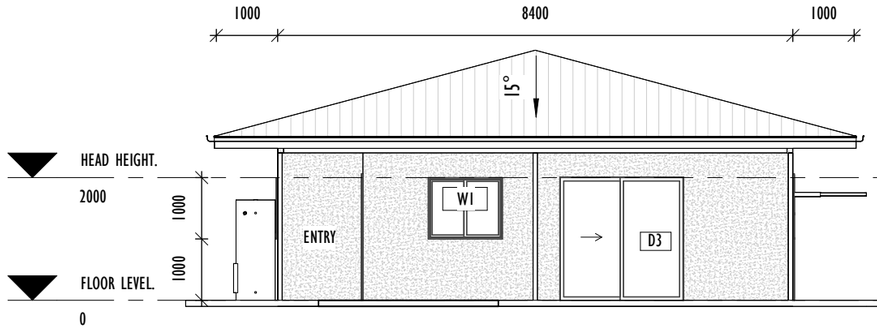

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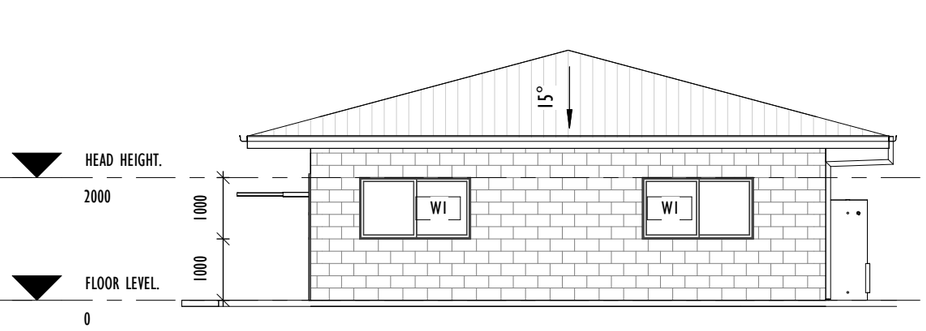

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REVIEW

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REV.	DESCRIPTION	DATE
R1	REVIEW	14.05.26

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MYRIAD	STUART JACKSON	14.05.26

HOPE VALE

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a. 176 buchan street, cairns qld 4870
t. 07 4041 7258 QBSA 1164388

PROJECT NAME:
SECOND DWELLING

PROJECT ADDRESS:
121 LINK RD, HOPEVALE

CLIENT:
HVASC

ELEVATIONS

SCALE	1 : 100@ A3
ISSUE DATE	14.05.26

SHEET NUMBER 3007 - 7.1

APPENDIX C

25 FLIERL STREET STORMWATER CALCULATIONS

CATCHMENT HYDROLOGY
(RATIONAL METHOD)

PROJECT No.

CALCULATION BY DATE

CHECKED BY DATE

SHEET OF

Project Name:

Project Location:

(1) Guidelines

- Queensland Urban Drainage Manual (QUDM) 2017
- Bureau of Meteorology (BOM)

Flierl St Pre Catchment Area

Input
Output

(2) Time of Concentration (t_c)

a) Overland Flow (Friend's equation)

$t = (107\ n\ L^{0.333}) / S^{0.2}$

where:

t = overland sheet flow travel time (min)
L = overland sheet flow path length (m)
n = Horton's surface roughness factor
S = slope of surface (%)

L (m) =	85.0
U/S RL (m) =	49.83
D/S RL (m) =	48.17
S (%) =	1.95
n =	0.0350

- refer QUDM 2017, Figure 4.4, assumed poorly grassed

t (mins) = **14.4**

b) Total tc

t_c (mins) = **14.4**

(3) Design Flow (Q)

$Q_y = 0.00278 \times C_y \times {}^{t}I_y \times A$ *- refer QUDM 2017, Section 4.3*

where:

Q_y = peak flow rate (m^3/s) for annual exceedence probability (AEP) of 1 in 'y' years
C_y = coefficient of discharge (dimensionless) for AEP of 1 in 'y' years
A = area of catchment (ha)
${}^{t}I_y$ = average rainfall intensity (mm/h) for a design duration of 't' hours and an AEP of 1 in 'y' years
t = the nominal design storm duration as defined by the time of concentration (t_c)

Design ARI, y =	100			
A (ha) =	0.2225	=	2225	m2
t_c (mins) =	14.4		Catchment Area (m2) =	2225
Rainfall intensity, I (mm/hr) =	50.7		Impervious Area (m2) =	250
${}^{1}I_{10}$ (mm/hr) =	82.1		Fraction Impervious =	0.11
Fraction impervious, f_i =	0.11	*- refer QUDM 2017, Table 4.5.1*		
Frequency Factor, F =	1.20	*- refer QUDM 2017, Table 4.5.2*		
C_{10} =	0.72	*- refer QUDM 2017, Table 4.5.3 & 4.5.4*		
C_y =	0.87			

Q (m^3/s) = **0.027**

CATCHMENT HYDROLOGY
(RATIONAL METHOD)

PROJECT No.

CALCULATION BY DATE

CHECKED BY DATE

SHEET OF

Project Name:

Project Location:

(1) Guidelines

- Queensland Urban Drainage Manual (QUDM) 2017
- Bureau of Meteorology (BOM)

Flierl St Post Catchment Area

Input
Output

(2) Time of Concentration (t_c)

a) Overland Flow (Friend's equation)

$t = (107 \, n \, L^{0.333}) / S^{0.2}$

where:

t = overland sheet flow travel time (min)
L = overland sheet flow path length (m)
n = Horton's surface roughness factor
S = slope of surface (%)

L (m) =	85.0
U/S RL (m) =	49.83
D/S RL (m) =	48.17
S (%) =	1.95
n =	0.0350

- refer QUDM 2017, Figure 4.4, assumed poorly grassed

t (mins) = **14.4**

b) Total tc

t_c (mins) = **14.4**

(3) Design Flow (Q)

$Q_y = 0.00278 \times C_y \times {}^{t}I_y \times A$ *- refer QUDM 2017, Section 4.3*

where:

Q_y = peak flow rate (m^3/s) for annual exceedence probability (AEP) of 1 in 'y' years
C_y = coefficient of discharge (dimensionless) for AEP of 1 in 'y' years
A = area of catchment (ha)
${}^{t}I_y$ = average rainfall intensity (mm/h) for a design duration of 't' hours and an AEP of 1 in 'y' years
t = the nominal design storm duration as defined by the time of concentration (t_c)

Design ARI, y =	100			
A (ha) =	0.2225	=	2225	m2
t_c (mins) =	14.4		Catchment Area (m2) =	2225
Rainfall intensity, I (mm/hr) =	50.7		Impervious Area (m2) =	578
${}^{1}I_{10}$ (mm/hr) =	82.1		Fraction Impervious =	0.26
Fraction impervious, f_i =	0.26	*- refer QUDM 2017, Table 4.5.1*		
Frequency Factor, F =	1.20	*- refer QUDM 2017, Table 4.5.2*		
C_{10} =	0.75	*- refer QUDM 2017, Table 4.5.3 & 4.5.4*		
C_y =	0.90			

Q (m^3/s) = **0.028**

APPENDIX D

121 LINK ROAD STORMWATER CALCULATIONS

CATCHMENT HYDROLOGY
(RATIONAL METHOD)

PROJECT No.

CALCULATION BY DATE

CHECKED BY DATE

SHEET OF

Project Name:

Project Location:

(1) Guidelines

- Queensland Urban Drainage Manual (QUDM) 2017
- Bureau of Meteorology (BOM)

Link St Pre Catchment Area

Input
Output

(2) Time of Concentration (t_c)

a) Overland Flow (Friend's equation)

$t = (107\ n\ L^{0.333}) / S^{0.2}$

where:

t = overland sheet flow travel time (min)
L = overland sheet flow path length (m)
n = Horton's surface roughness factor
S = slope of surface (%)

L (m) =	496.0
U/S RL (m) =	53.99
D/S RL (m) =	44.95
S (%) =	1.82
n =	0.0450

- refer QUDM 2017, Figure 4.4, assumed average grassed

t (mins) = **33.7**

b) Total tc

t_c (mins) = **33.7**

(3) Design Flow (Q)

$Q_y = 0.00278 \times C_y \times {}^{t}I_y \times A$ *- refer QUDM 2017, Section 4.3*

where:

Q_y = peak flow rate (m^3/s) for annual exceedence probability (AEP) of 1 in 'y' years
C_y = coefficient of discharge (dimensionless) for AEP of 1 in 'y' years
A = area of catchment (ha)
${}^{t}I_y$ = average rainfall intensity (mm/h) for a design duration of 't' hours and an AEP of 1 in 'y' years
t = the nominal design storm duration as defined by the time of concentration (t_c)

Design ARI, y =	100			
A (ha) =	6.7091	=	67091	m2
t_c (mins) =	33.7		Catchment Area (m2) =	67091
Rainfall intensity, I (mm/hr) =	82.8		Impervious Area (m2) =	3055
${}^{1}I_{10}$ (mm/hr) =	82.1		Fraction Impervious =	0.05
Fraction impervious, f_i =	0.05	*- refer QUDM 2017, Table 4.5.1*		
Frequency Factor, F =	1.20	*- refer QUDM 2017, Table 4.5.2*		
C_{10} =	0.71	*- refer QUDM 2017, Table 4.5.3 & 4.5.4*		
C_y =	0.85			

Q (m^3/s) = **1.313**

CATCHMENT HYDROLOGY
(RATIONAL METHOD)

PROJECT No.

CALCULATION BY DATE

CHECKED BY DATE

SHEET OF

Project Name:

Project Location:

(1) Guidelines

- Queensland Urban Drainage Manual (QUDM) 2017
- Bureau of Meteorology (BOM)

Link St Post Catchment Area

Input
Output

(2) Time of Concentration (t_c)

a) Overland Flow (Friend's equation)

$t = (107\ n\ L^{0.333}) / S^{0.2}$

where:

t = overland sheet flow travel time (min)
L = overland sheet flow path length (m)
n = Horton's surface roughness factor
S = slope of surface (%)

L (m) =	496.0
U/S RL (m) =	53.99
D/S RL (m) =	44.95
S (%) =	1.82
n =	0.0450

- refer QUDM 2017, Figure 4.4, assumed average grassed

t (mins) = **33.7**

b) Total tc

t_c (mins) = **33.7**

(3) Design Flow (Q)

$Q_y = 0.00278 \times C_y \times {}^{t}I_y \times A$ *- refer QUDM 2017, Section 4.3*

where:

Q_y = peak flow rate (m^3/s) for annual exceedence probability (AEP) of 1 in 'y' years
C_y = coefficient of discharge (dimensionless) for AEP of 1 in 'y' years
A = area of catchment (ha)
${}^{t}I_y$ = average rainfall intensity (mm/h) for a design duration of 't' hours and an AEP of 1 in 'y' years
t = the nominal design storm duration as defined by the time of concentration (t_c)

Design ARI, y =	100			
A (ha) =	6.7091	=	67091	m2
t_c (mins) =	33.7		Catchment Area (m2) =	67091
Rainfall intensity, I (mm/hr) =	82.8		Impervious Area (m2) =	3472
${}^{1}I_{10}$ (mm/hr) =	82.1		Fraction Impervious =	0.05
Fraction impervious, f_i =	0.05	*- refer QUDM 2017, Table 4.5.1*		
Frequency Factor, F =	1.20	*- refer QUDM 2017, Table 4.5.2*		
C_{10} =	0.71	*- refer QUDM 2017, Table 4.5.3 & 4.5.4*		
C_y =	0.85			

Q (m^3/s) = **1.316**

APPENDIX E

VENANAT SOLUTIONS - 25 FLIERL STREET FLOOD ADVICE

Venant Solutions Pty Ltd

Office 704, 222 Hoddle Street
Abbotsford VIC 3067

PO Box 877
Macleod VIC 3085

Ph: 03 9089 6700
ABN: 15 166 193 219

www.venantsolutions.com.au

Our Ref: L.M00605.002.00.docx

04 June 2026

Langtree Consulting Pty Ltd
Level 1, 14 Ingham Road
West End QLD 4810

Attention: Mr Brett Langtree

Dear Brett

RE: Flood Level Advice - 25 Flierl Street, Hope Vale

The proposed house footprint is in the northwest corner of the lot as shown on the attached proposed layout plan. Data from the Hope Vale Flood Study indicates that this lot is not subject to flooding, but there will be shallow overland flow with maximum depth of 50 mm. Based on the LiDAR data this gives a flood level of 48.7 m AHD. The attached figure shows the depth mapping at and surrounding the lot.

Please do not hesitate to contact me should you require any further information.

Yours sincerely,

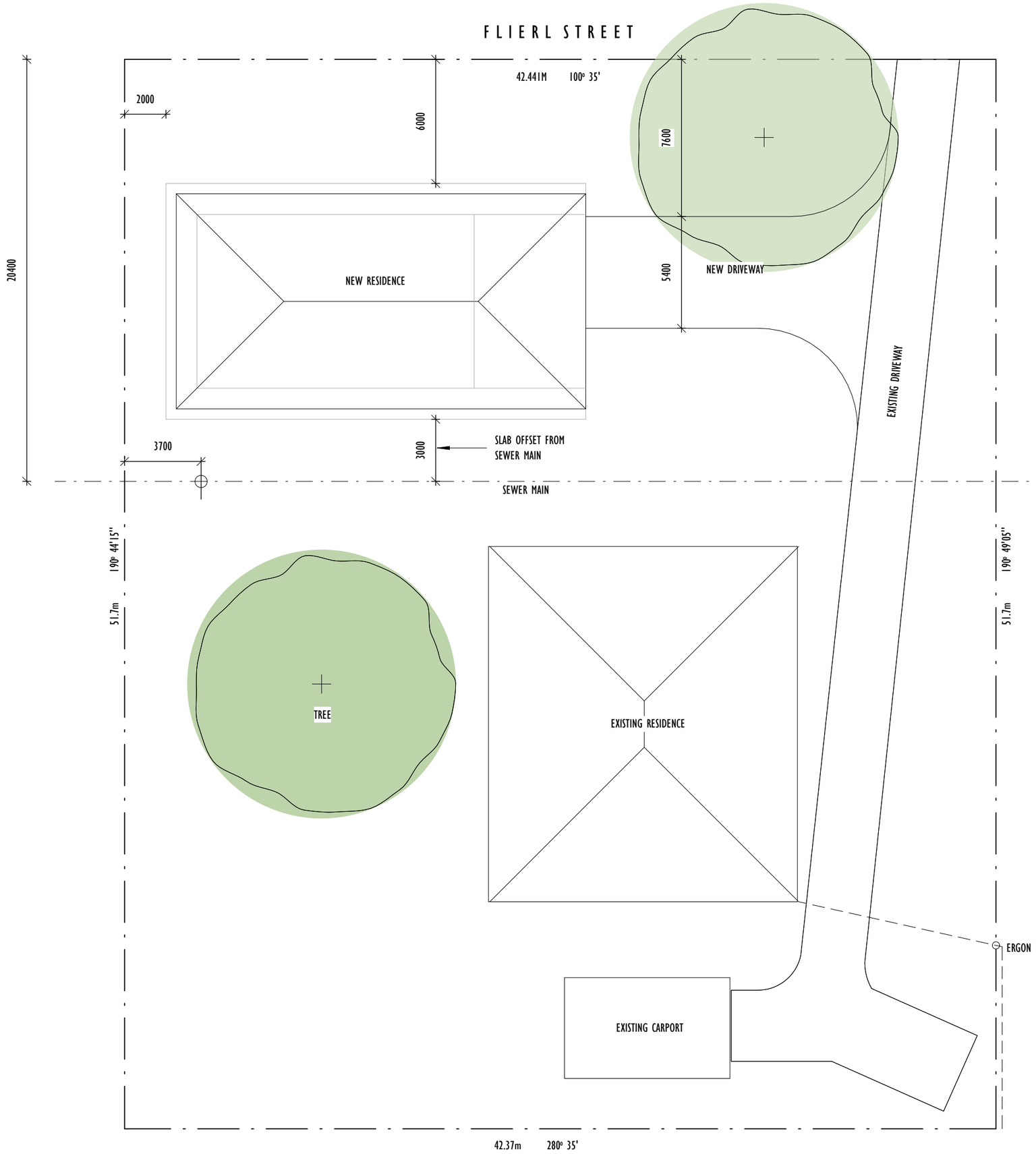
Theo Liu

Project Engineer

Dr Mark Jempson

Director (RPEQ 11111)

Title: Hope Vale Flood Study - 25 Flierl Street Existing Case Flood Depth - 1% AEP	Figure: 1	Rev: A	0 10 20 m	This mapping product is based on techniques and data in accordance with the study scope. Users should consider the mapping in the context of the report. No two floods are the same and care should be taken in the use and interpretation of the results presented.	VENANT SOLUTIONS www.venantsolutions.com.au
Filename: S:\Projects\M00605.MJ.HopeValeFloodStudy\GIS\Drawings\L.M00605.002.00\Fig1_HVFS_Near-term_SSP5-8.5_100y_005_d_MaxMax.qgz				By: TL	Jun 2026



REVIEW

DRAWING STATUS:

REV.	DESCRIPTION	DATE

DRAWN BY:	CHECKED BY:	CHECKED ON:
MYRIAD	STUART JACKSON	21.04.26

myriad
tapitor pty ltd trading as myriad design abn 23 055 558 659
a. 176 buchan street, cairns qld 4870
t. 07 4041 7258 QBSA 1164388

PROJECT NAME:
NEW 2 BED HOUSE

PROJECT ADDRESS:
25 FLIERL ST, HOPEVALE

CLIENT:
HVASC

SITE PLAN

SCALE 1 : 200@ A3
ISSUE DATE 21.04.26

SHEET NUMBER 3007 - 2.1

APPENDIX F

VENANAT SOLUTIONS - 121 LINK ROAD FLOOD ADVICE

Venant Solutions Pty Ltd

Office 704, 222 Hoddle Street
Abbotsford VIC 3067

PO Box 877
Macleod VIC 3085

Ph: 03 9089 6700
ABN: 15 166 193 219

www.venantsolutions.com.au

Our Ref: L.M00605.003.00.docx

04 June 2026

Langtree Consulting Pty Ltd
Level 1, 14 Ingham Road
West End QLD 4810

Attention: Mr Brett Langtree

Dear Brett

RE: Flood Level Advice - 121 Link Road, Hope Vale

The proposed house footprint is on the south side of the lot as shown on the attached proposed layout plan. Data from the Hope Vale Flood Study indicates that this lot is subject to flooding coming on to the lot from the west. At the location of the proposed house the flood level is 48.05 m AHD with typical depth of about 300 mm. The attached figure shows the depth mapping at and surrounding the lot.

Please do not hesitate to contact me should you require any further information.

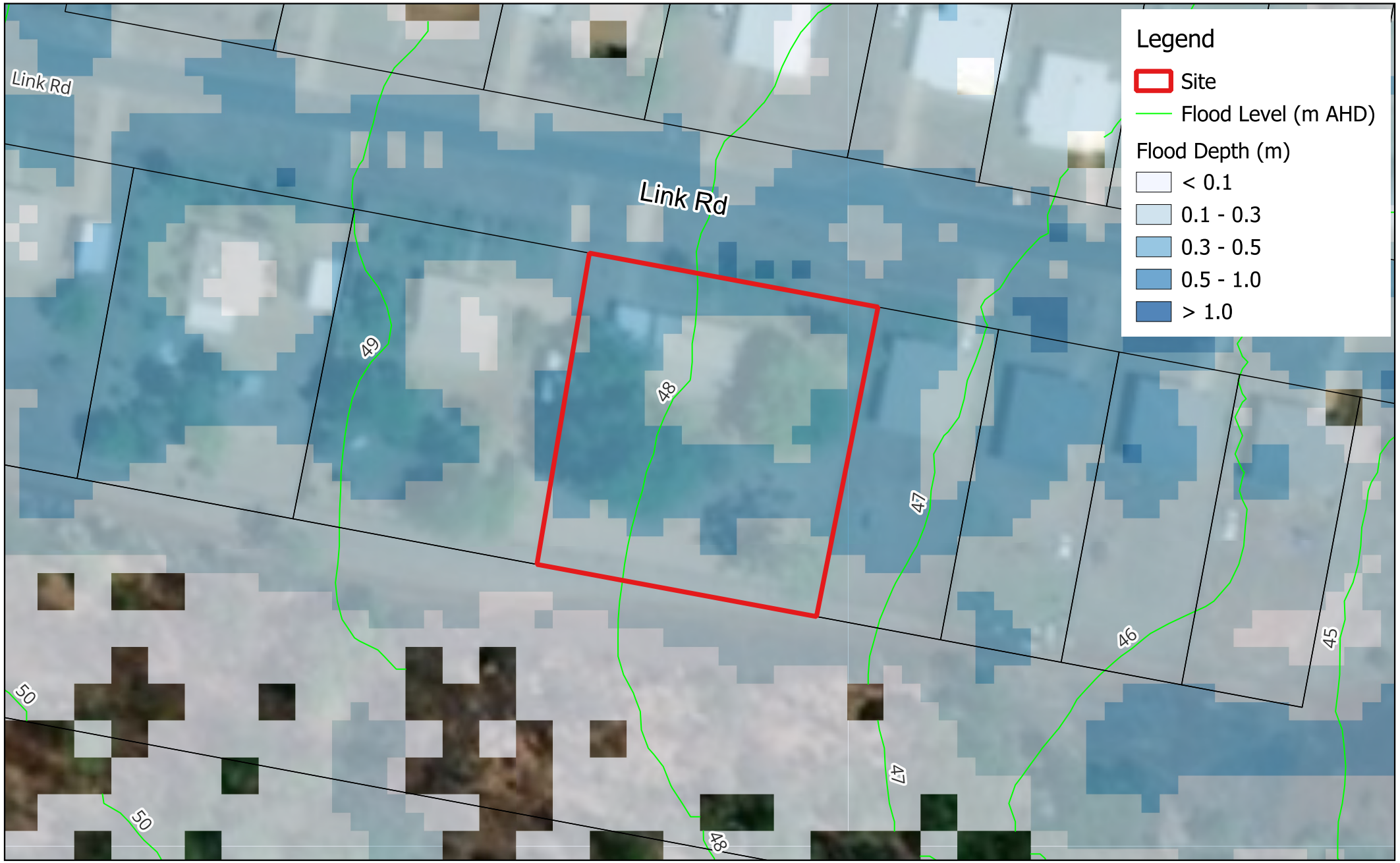
Yours sincerely,

Theo Liu

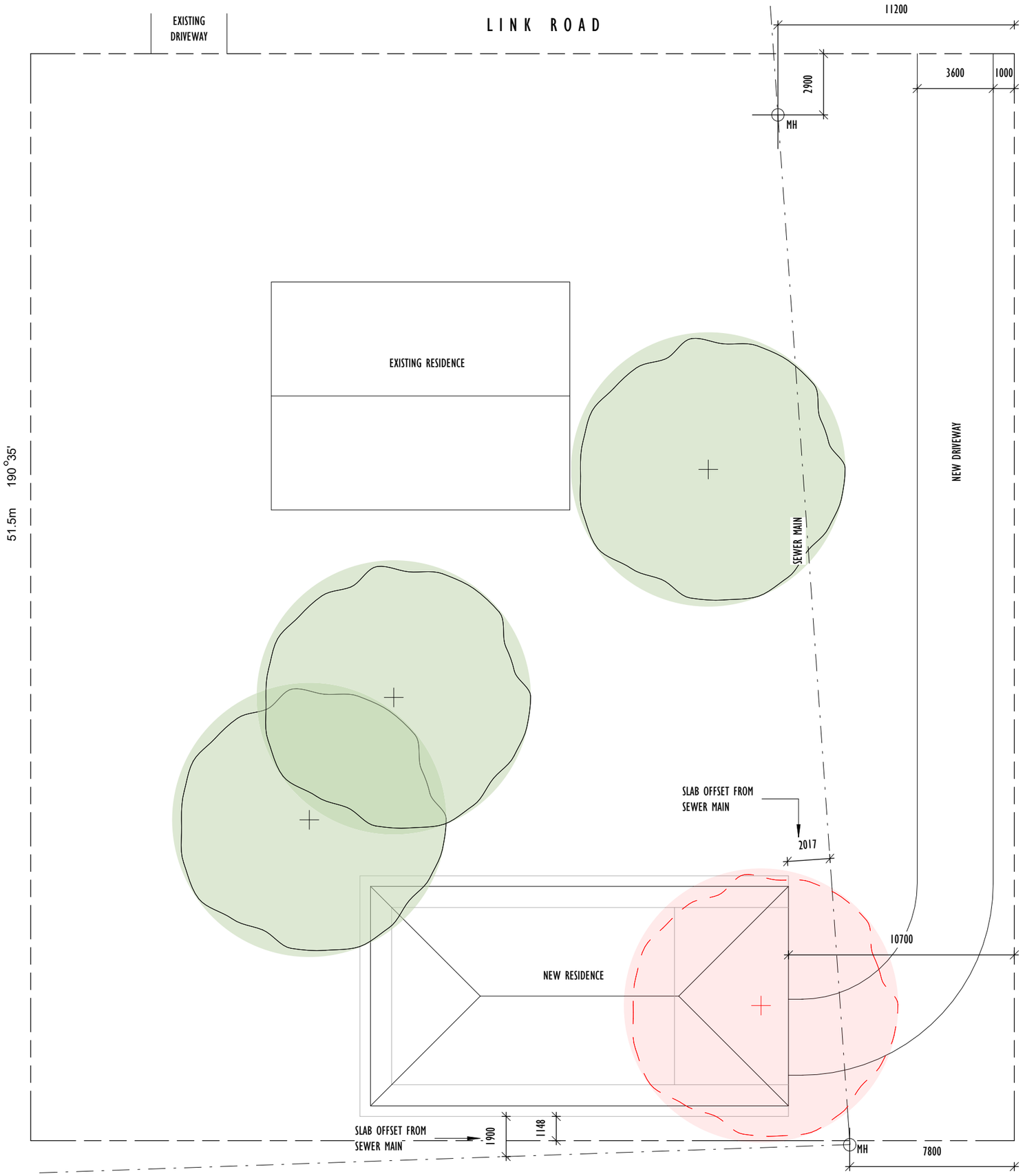
Project Engineer

Dr Mark Jempson

Director (RPEQ 11111)



Title: Hope Vale Flood Study - 121 Link Road Existing Case Flood Depth - 1% AEP	Figure: 1	Rev: A	0 10 20 m	This mapping product is based on techniques and data in accordance with the study scope. Users should consider the mapping in the context of the report. No two floods are the same and care should be taken in the use and interpretation of the results presented.	VENANT SOLUTIONS www.venantsolutions.com.au
Filename: S:\Projects\M00605.MJ.HopeValeFloodStudy\GIS\Drawings\L.M00605.003.00\Fig1_HVFS_Near-term_SSP5-8.5_100y_005_d_MaxMax.qgz				By: TL	Jun 2026



REVIEW

DRAWING STATUS:

REV.	DESCRIPTION	DATE

DRAWN BY:	CHECKED BY:	CHECKED ON:
MYRIAD	STUART JACKSON	21.04.26

myriad
tapitor pty ltd trading as myriad design abn 23 055 558 659
a. 176 buchan street, cairns qld 4870
t. 07 4041 7258 QBSA 1164388

PROJECT NAME:
NEW 2 BED HOUSE

PROJECT ADDRESS:
121 LINK ROAD

CLIENT:
HVASC

SITE PLAN

SCALE 1 : 200@ A3
ISSUE DATE 21.04.26

SHEET NUMBER 3005 - 2.1